




Engineered Wood Trusses

Understanding and Installing Trusses:

Definition

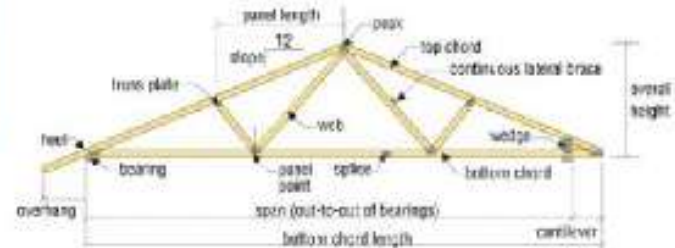
- ▶ A **truss** is a frame comprising one or more triangular units constructed with straight members whose ends are connected at joints referred to as nodes. External forces and reactions to those forces are considered to act only at the nodes and result in forces in the members which are either tensile or compressive forces.
- 

Truss Technology in Building

TRUSS CONFIGURATIONS

The following examples represent some of the possible variations on the basic types of trusses.

Click on graphics to learn more.



SCISSORS



POLYNESIAN



CLERESTORY



MONO



STANDARD ROOF TRUSSES



VAULTED PARALLEL CHORD



GAMBREL



CANTILEVERED MANSARD W/PARAPETS



HALF SCISSORS



DOUBLE FINK



VAULT



ROOM-IN-ATTIC



HIP



HALF HIP



DOUBLE HOWE



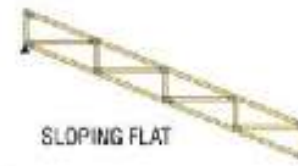
FLAT VAULT



BOWSTRING



DUAL PITCH



SLOPING FLAT



MODIFIED FAN



STUDIO VAULT



DOUBLE CANTILEVER



DOUBLE INVERTED



TRAY OR COFFER



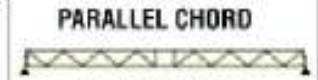
TRI-BEARING



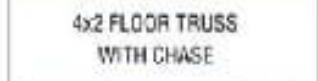
MULTI-PIECE/PIGGYBACK



BARREL VAULT



PARALLEL CHORD




4x2 FLOOR TRUSS WITH CHASE



2x4 FLOOR OR ROOF TRUSS


Importance

- ▶ Wood Roof Trusses are the most common roof framing system employed in small buildings in Ontario
 - ▶ Wood Floor Trusses are often used on projects where long clear spans are required
 - ▶ Wood trusses, along with walls, floors and foundations are one of the major structural parts of most small buildings
- 


Trusses in the Building Code

- The NBC mentions trusses in few places
- 9.23.5.5. Roof Trusses
 - shall not be notched, drilled or otherwise weakened
- 9.23.13.11. Wood Roof Trusses
 - if not designed under Pt.4, deflection limits, 6' web bracing, method of testing, method of analysis
- 9.4.3.1 (Deflection - $L/360$, $L/240$), 9.23.3.4 (Nailing - 3 Toenails-3.25"), 9.23.10.7 (Studs under girders)

Code Based Checklist

1. Damage to truss?.....easy
 2. 6' compression webs braced ??????
 3. TPIC design ???????
 4. Deflection ???????
 5. Nailing at Bearing?.....easy
 6. Studs under girders?...easy
- 


How to Check the Other Items

- ▶ Every manufactured truss used in Ontario must have an Ontario Engineer stamped drawing available for the Builder
 - ▶ The Builder should ask for the truss and layout drawings on every project – make sure you have the Drawings when the trusses are delivered
 - ▶ On simple projects you still need the truss drawings to determine web bracing
 - ▶ Use the Engineering Drawings and Truss Layout Supplied by the Truss manufacturer to check design and installation requirements
- 

Truss Engineering Drawings

- ▶ Contain a lot of info, some important to the builder, some not
- ▶ Examples of truss drawings from the 2 main suppliers of truss connector plates and truss design software
 - Mitek
 - Alpine

Important Info on Drawings

- ▶ Loads and Design Basis – Part 4, Part 9
 - ▶ Spacing
 - ▶ Deflection
 - ▶ Support locations
 - ▶ Web and chord bracing
 - ▶ Uplift, bearing and hanger requirements
- 

Loads and Design

Standard Part 9 Loads:

- no attic trusses
- no spans over 40'

Eg: Live Load Due to Snow
 $= 0.55 \times 48.1 \text{ psf} + 6.3 = 32.7 \text{ psf}$

Part 4 Loads:

- Part 9 over 40', Attic trusses
- Post Disaster Buildings
- Part 4 Buildings

Eg: Live Load Due to Snow
 $= 0.8 \times 48.1 + 6.3 = 44.8 \text{ psf}$

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED
 BY USER
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH LL = 32.7 PSF
 DL = 7.0 PSF
 BOT CH LL = 10.0 PSF
 DL = 10.0 PSF
 TOTAL LOAD = 59.7 PSF
 SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
 SLOPE OF 6.0W/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO
 ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
 OR SMALL BUILDING REQUIREMENTS OF
 PART 9, NBCC 2005

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2006 , BCBC 2006 , ABC 2006
 - CSA 086-01
 - TRC 2007

(55 % OF 48.1 P.S.F. G.S.L. PLUS
 6.3 P.S.F. RAIN LOAD EQUALS
 32.7 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. = L/360 (1.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI. TC=0.35 (3-4:1) , BC=0.55 (16-18:1) ,
 WB=0.97 (6-15:1) , SSI=0.94 (16-16:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

Design Crit: RESIDENTIAL

7.31.1205.21

QTY:1 ON/1/1/1/1/1/1

THIS DRAWING MUST BE APPROVED BY THE BUILDING DESIGNER AND REVIEWED BY THE TRUSS INSTALLER BEFORE USE. VISIT <http://alpinetys.com/Specs> FOR THE LATEST INFORMATION AND WARNINGS AND SEE A100 FOR GENERAL NOTES. IMPORTANT SPECIFICATIONS AND WARNINGS CCMC #12182-L 12802-L 13112-L CONFORMS TO NBCC 2005 PART 9 HOUSING AND SMALL BUILDINGS (LSD)
 Ground Snow Load = 48.1 psf OR LESS
 Rain Load = 6.3 psf OR LESS
 Cb = 0.55 Cw = 1.00 Cs = 1.00 Importance Factor = 1.00



TC LL	32.7 PSF
TC DL	7.0 PSF
BC DL	7.0 PSF
BC LL	10.0 PSF
TOT. LD.	52.7 PSF
DUR. FAC.	1.00
SPACING	24.0"

Spacing

Design Crit: RESIDENTIAL

7.31.1205.21

QTY:1 ON/-/1/-/-/-/-

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(LSD)

Ground Snow Load = 48.1 psf OR LESS

Rain Load = 6.3 psf OR LESS

$C_b = 0.55$ $C_w = 1.00$ $C_s = 1.00$ Importance Factor = 1.00



TC LL	32.7 PSF
TC DL	3.0 PSF
BC DL	7.0 PSF
BC LL	10.0 PSF
TOT. LD.	52.7 PSF
DUR. FAC.	1.00
SPACING	24.0"

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LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH	LL = 32.7 PSF
	DL = 7.0 PSF
BOT CH	LL = 10.0 PSF
	DL = 10.0 PSF
TOTAL LOAD	59.7 PSF

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2005

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2006, BCBC 2006, ABC 2006
- CSA D86-01
- TPIC 2007

(55 % OF 48.1 P.S.F. G.S.L. PLUS 6.3 P.S.F. RAIN LOAD EQUALS 32.7 P.S.F. SPECIFIED ROOF LIVE LOAD

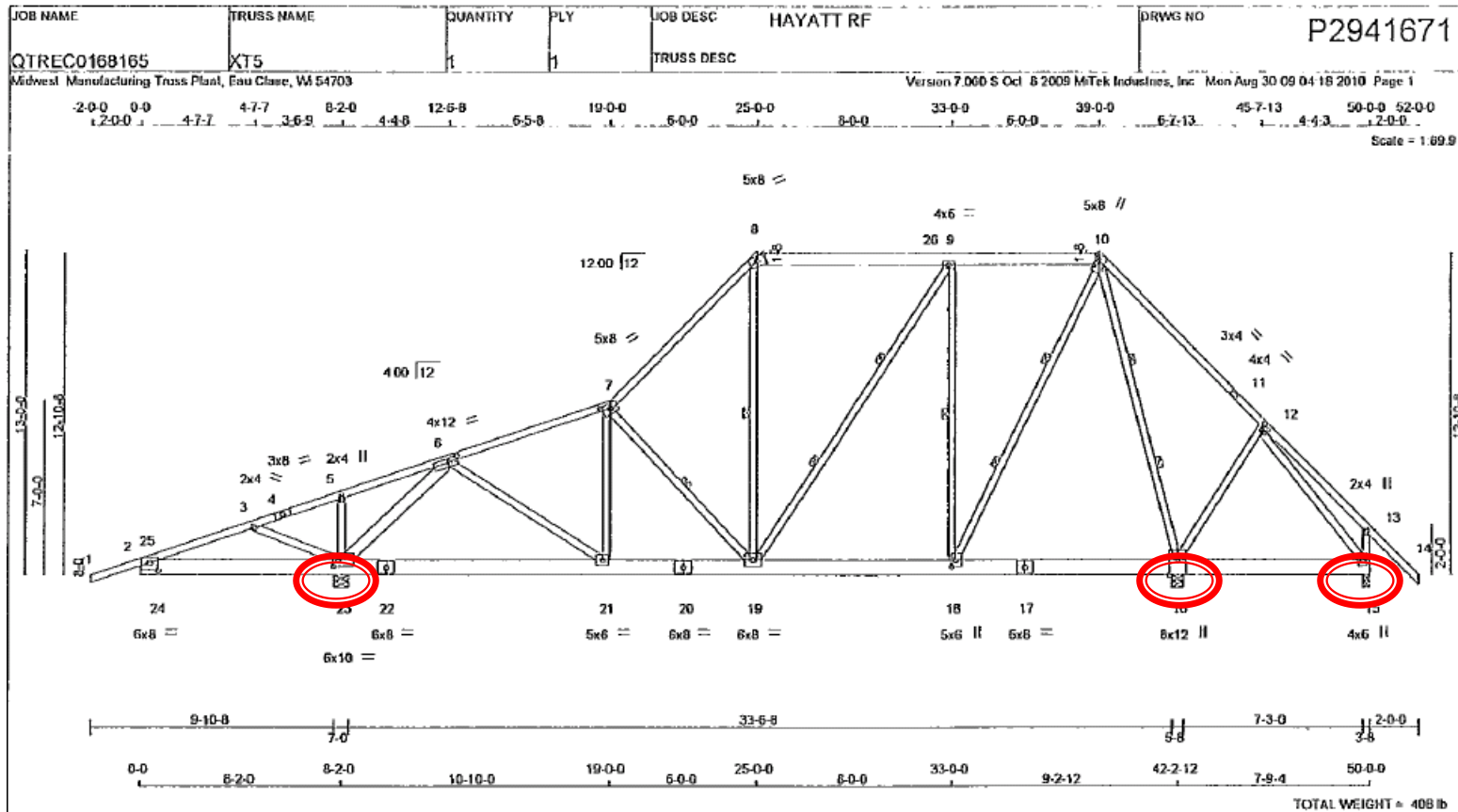
ALLOWABLE DEFL. = $L/360$ (1.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.13")

CSI: TC=0.36 (3-4:1), BC=0.55 (16-18:1),
WB=0.97 (6-15:1), SSI=0.94 (16-16:1)

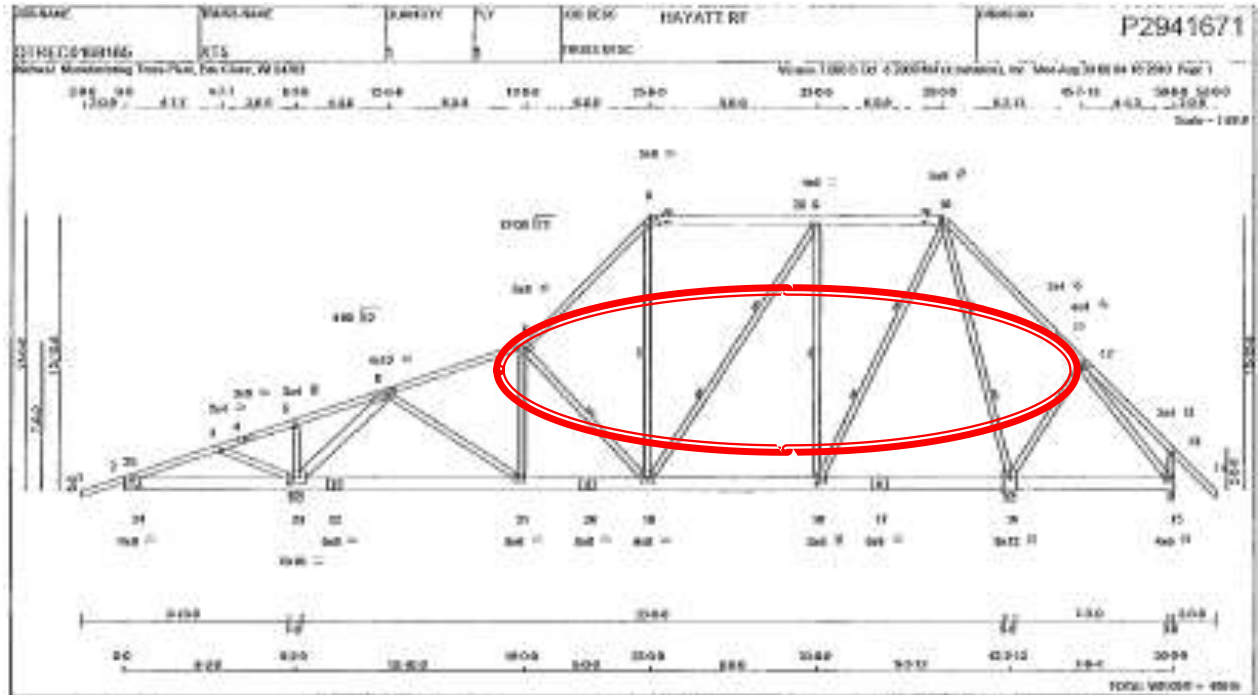
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

Support Locations



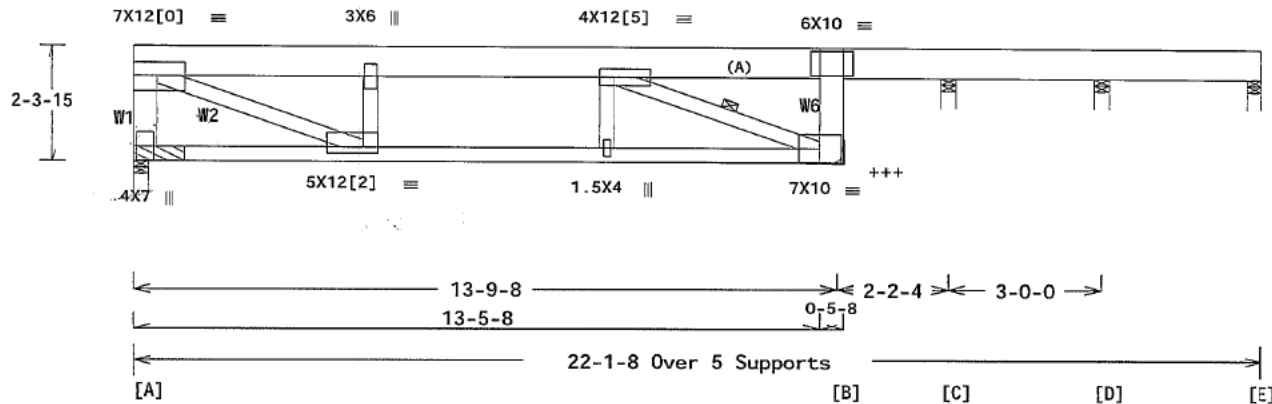
Web Bracing



LEN	Y	X	ID	033	3157 U	307 U	447 -103	1907 U	LRWD/UNCLD(p)	
8.0	3.75	4.00	BEARING MATERIAL TO BE OF SPF NO. 2 OR BETTER							THIS TRUSS IS OR INDUSTRIAL PART 4, NBCC
4.0			BRACING							THIS DESIGN C - PART 4 OF OE - CSA 086-01 - TPIC 2007
8.0			MAX. UNBRACED TOP CHORD LENGTH = 2.88FT.							
4.0			MAX. UNBRACED BOTTOM CHORD LENGTH = 5.21FT. OR RIGID CEILING DIRECTLY APPLIED							DESIGN ASSUM - SLOPE REDU
12.0	1.75	4.25	1-2X4 LATERAL BRACE REQUIRED AT 1/2 LENGTH OF 7-19, 8-19, 9-16							(80% OF 48.1 F 6.3 P.S.F RAIN FACTOR EQU/ 44.7 P.S.F SPI
8.0	2.25	4.00	2-2X4 LATERAL BRACE REQUIRED AT 1/3 LENGTH OF 9-19, 10-18, 10-16							
8.0	Edge	4.00	END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW							ALLOWABLE DI CALCULATED V
6.0			LOADING							CANTILEVER I
8.0			LOAD CASE (1) OF (17)							
4.0	2.00	1.75	CHORDS				WEBS			
4.0			FACTORED	FACTORED			FACTORED			
6.0	3.00	2.00	MEMB. FORCE (LBS)	VERT LOAD (PLF)	MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX CSI (LC)		
8.0			FR-TO	FROM TO			FR-TO			
6.0	Edge	4.00								

Web Bracing - Girders or Single Trusses

(A) 2x4 SPF#3 CLB's EQUALLY SPACED ON MEMBER. ATTACH W/(2) 3.5" NAILS. OR 2x4 "T" BRACE. 80% LENGTH OF WEB MEMBER. SAME SPECIES & GRADE OR BETTER, ATTACHED W/3.0" NAILS @ 6" OC. BRACING MATERIAL SUPPLIED & ATTACHED @ BOTH ENDS TO SUITABLE SUPPORT BY ERECTION CONTRACTOR. DESIGN OF PERMANENT BUILDING BRACING SYSTEM BY PROJECT ENGINEER OF RECORD.



Web Bracing- Gable End

(25019-/WEBB ROOF -- , ** - T23 24' DTC Gable)

TOP CHORD 2x4 SPF No.1/No.2
 BOT CHORD 2x4 SPF No.1/No.2
 WEBS 2x4 SPF No.1/No.2 :W2, W5 2x4 SPF 2100r-1.8E:

MAX CSI: TC = 0.18, BC = 0.07, WEBS = 0.24.

SEE A-101, NOTE 3 FOR STANDARD PLATE POSITIONING.

PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.

2^{5x} PLATE POSITIONING TOLERANCE.

THIS DRAW PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

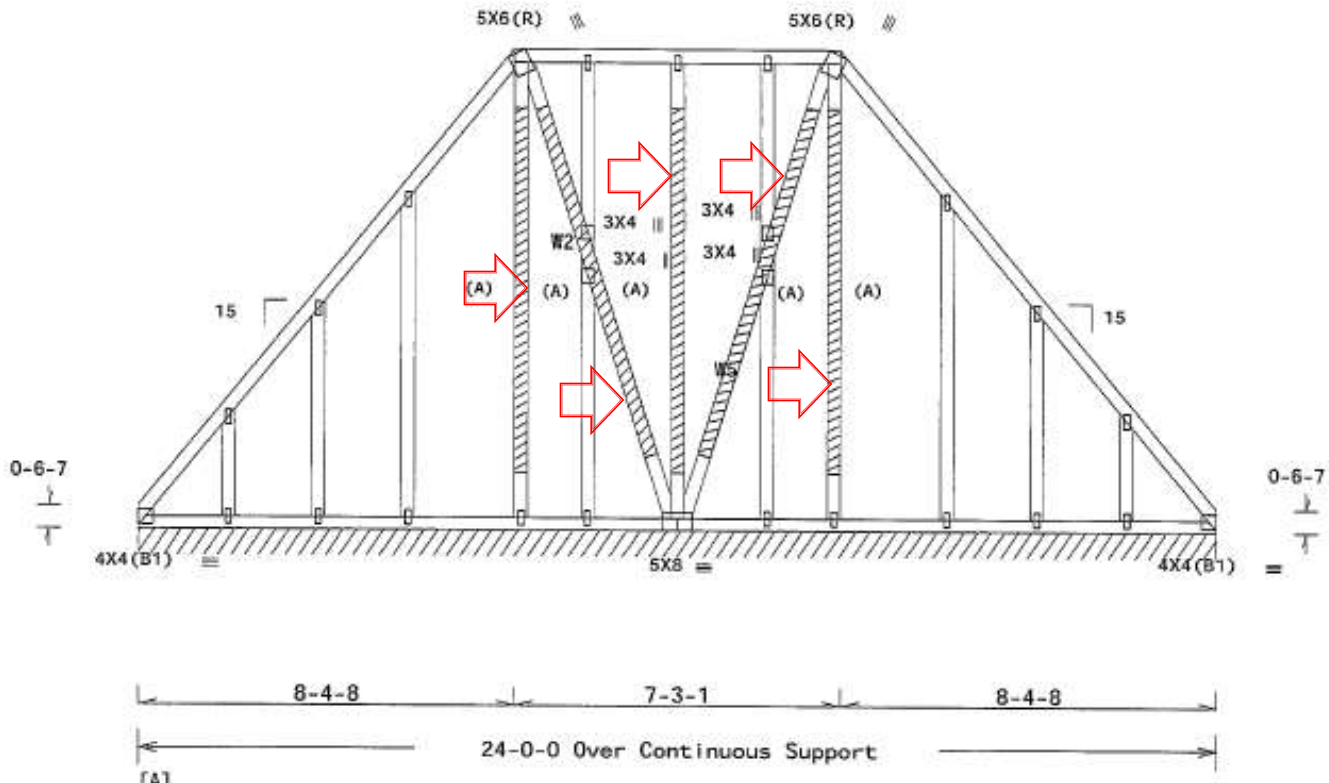
Loc	H	W	L	A	M	R	S	T	U	V
A 0'	17	3	12	24'	1570	3360	1337	6032	0	

(A) #3 OR BETTER SCAB BRACE, SAME SIZE & 80% LENGTH OF WEB MEMBER. ATTACH W/3.5 NAILS @ 6" OC. BRACING MATERIAL SUPPLIED BY ERECTON CONTRACTOR.

IN LIEU OF STRUCTURAL PANELS USE PURLINS TO BRACE ALL FLAT TC @ 24.00" OC

LOADING SPEC'D BY ARCH. HAVING JURISDICTION & TYPE OF DESIGN.

PLATE SIZES INCREASED FOR 1.05X REQ'D AREA AND STEEL SECTION.



Chord Bracing

BRACING

MAX. UNBRACED TOP CHORD LENGTH = 2.88FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 5.21FT. OR RIGID CEILING DIRECTLY APPLIED.

1-2X4 LATERAL BRACE REQUIRED AT 1/2 LENGTH OF 7-19, 8-19, 9-18

2-2X4 LATERAL BRACE REQUIRED AT 1/3 LENGTH OF 9-19, 10-18, 10-16

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

IN LIEU OF STRUCTURAL PANELS USE PURLINS TO BRACE ALL FLAT TC @ 24.00" OC. PURLINS TO BE ATTACHED TO PERMANENT BUILDING BRACING. DESIGN OF PURLIN, PURLIN ATTACHMENT & PERMANENT BRACING SYSTEM AS PER PROJECT ENGINEER OF RECORD.

Uplift and Hanger Requirements

PROVIDE UPLIFT CONNECTION AT BEARINGS AS INDICATED
 FACTORED UPLIFT (LBS): 363 449 200 229 200
 BRG. LOC. (ft): 0.00 13.67 15.83 18.83 21.83
 q=10.00 psf, 12.66' REF.HT., CALC'D INT. PRESS=6.30 psf,
 C2 BLDG, 10.00/7.00 TCDL/BCDL. OPNGS MAINLY WINDWARD SIDE.

DEFLECTION MEETS L/360.00 LIVE LOAD AND L/360.00 TOTAL LOAD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

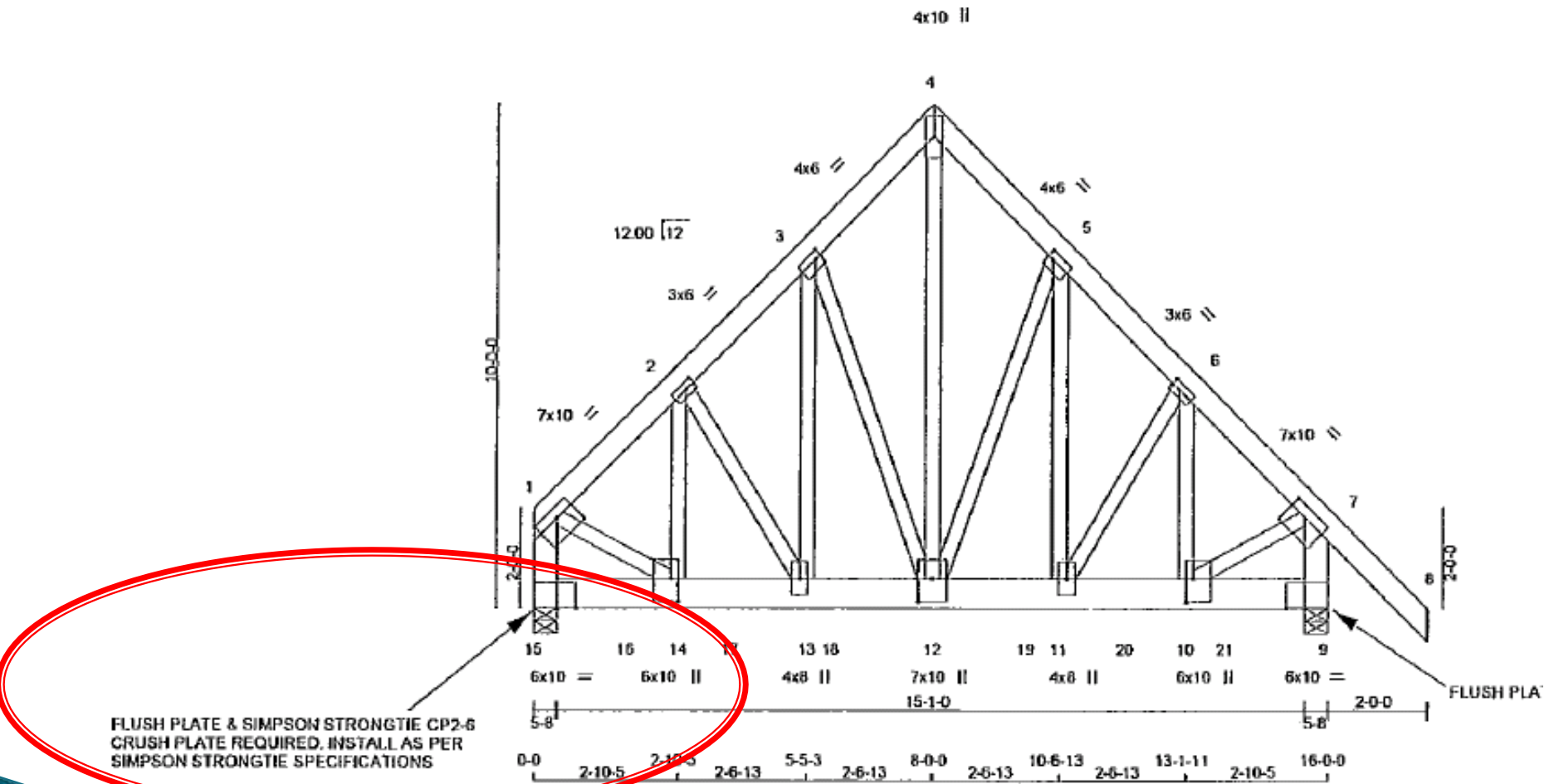
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-5X	IN-5X
20	4620	0	4620	0	0	5-8	4-5
11	4246	0	4246	0	0	HANGER BY OTHERS MIN. SEAT SIZE 3-0	

UNFACTORED GROSS REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	WIND	DEAD
20	3846	2052 / 0	671 / 0	0 / 0	1113 / 0
11	3385	1620 / 0	588 / 0	0 / 0	976 / 0

BEARING MATERIAL TO BE OF SPF NO. 2 OR BETTER

Bearing Conditions - Crush Plates



Bearing Conditions - Floor Required

ROOF -- , ** - T31 24' Double Slope Flat)

SPF No.1/No.2 :T2 2x6 SPF No.1/No.2:
 SPF No.1/No.2
 SPF No.1/No.2 :W1, W13 2x6 SPF No.1/No.2:

0.34, BC = 0.25, WEBS = 0.14.

TE 3 FOR STANDARD PLATE POSITIONING.

ED FOR FABRICATION USING SEASONED LUMBER.

CREASED FOR 1.05X REQ'D AREA AND STEEL SECTION.

LATERAL SHIFT	CHORD BITE
3.25 R	1.50
3.25 L	1.50
3.25 R	1.50
3.25 L	1.50

Loc	H	W	S	D	F	Hz
A 0'	9'4"10	5'8	623	230	290	1413 754
B 23'6"8	9'4"10	5'8	623	230	290	1413 754

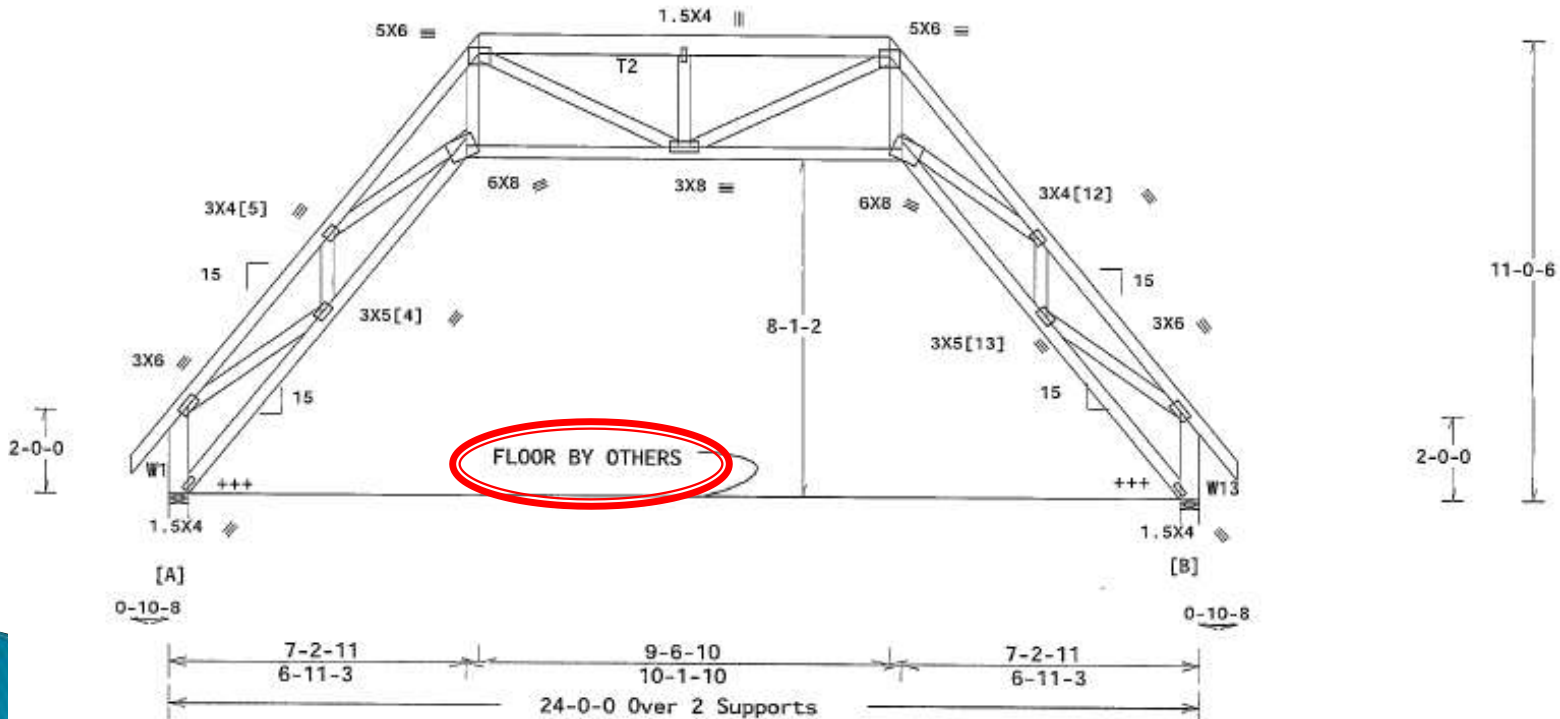
+++ TRUSSES TO BE PROPERLY ANCHORED AT SUPPORTS, BY OTHERS, TO WITHSTAND THE INDICATED VERTICAL AND HORIZONTAL REACTIONS.

DEFLECTION MEETS L/360.00 LIVE LOAD AND L/240.00 TOTAL LOAD.

IN LIEU OF STRUCTURAL PANELS USE PURLINS TO BRACE ALL FLAT TC @ 24.00" OC, PURLINS TO BE ATTACHED TO PERMANENT BUILDING BRACING. DESIGN OF PURLIN, PURLIN ATTACHMENT & PERMANENT BRACING SYSTEM AS PER PROJECT ENGINEER OF RECORD.

LOADING SPEC'D BY AUTH. HAVING JURISDICTION @ TIME OF DESIGN.

2sx PLATE POSITIONING TOLERANCE.

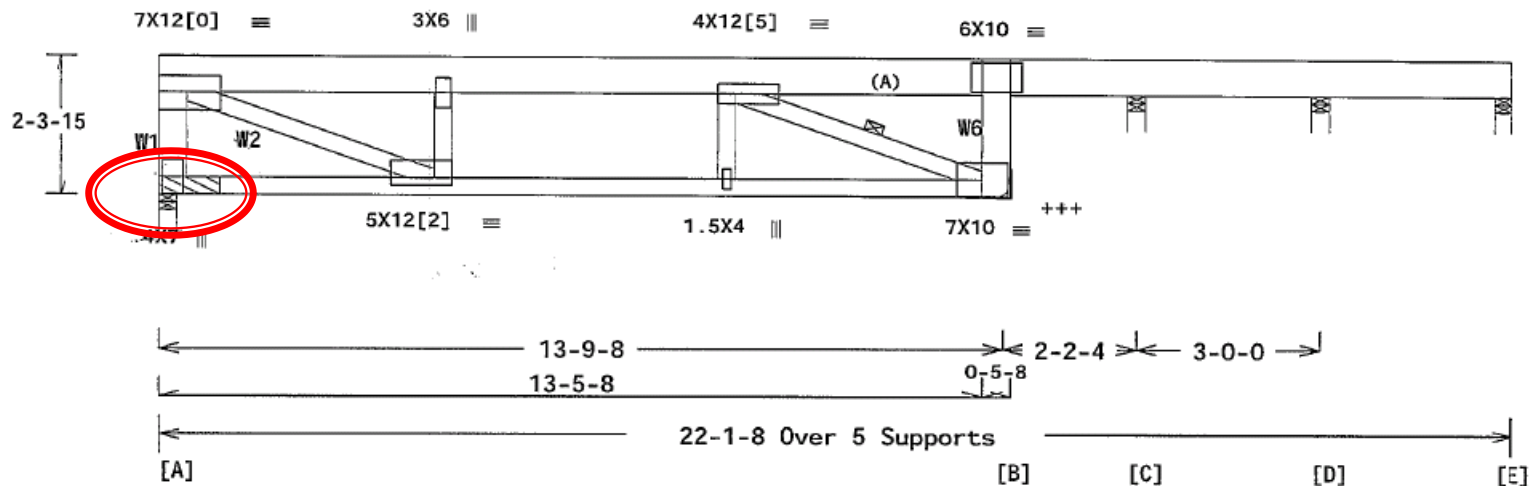


Bearing Conditions - Bearing Blocks Required

Bearing blocks: Nail type: 3.0" common nails

BRG	X-LOC	#BLOCKS	LENGTH/BLK	#NAILS/BLK
1	0.000'	1	12"	9

Bearing block to be same size and species as bottom chord.
Refer to drawing CNBRGBLK0207 for additional information.



Bearing Block Notes:

BEARING BLOCK NAIL SPACING DETAIL

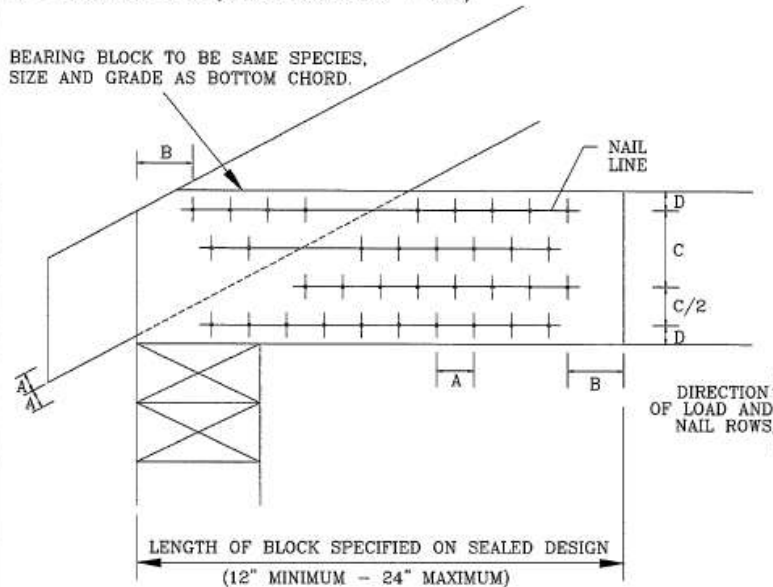
(SPF & NORTHERN SPECIES LUMBER)

VALUES FROM CSA 086-01 ENGINEERING DESIGN IN WOOD

MINIMUM SPACING FOR SINGLE BEARING BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

- A - SPACING PARALLEL TO GRAIN (16 NAIL DIAMETERS - MIN.)
- B - END DISTANCE (12 NAIL DIAMETERS - MIN.)
- C - SPACING OF NAILS IN A ROW (8 NAIL DIAMETERS - MIN.)
- D - EDGE DISTANCE (4 NAIL DIAMETERS - MIN.)

BEARING BLOCK TO BE SAME SPECIES, SIZE AND GRADE AS BOTTOM CHORD.



MAXIMUM NUMBER OF NAIL LINES PARALLEL TO GRAIN

NAIL TYPE	DIAM.	CHORD SIZE				
		2X4	2X6	2X8	2X10	2X12
2.5" COMMON NAIL	0.128"	5	8	11	15	18
3.0" COMMON NAIL	0.144"	4	7	10	13	17
3.5" COMMON NAIL	0.160"	3	6	9	12	15
2.5" SPIRAL NAIL	0.109"	6	11	15	19	24
3.0" SPIRAL NAIL	0.122"	6	10	13	17	21
3.5" SPIRAL NAIL	0.152"	4	7	10	13	17
3.25" GUN NAIL	0.128"	5	8	11	15	18

MINIMUM NAIL SPACING DISTANCES ++

NAIL TYPE	DIAM.	DISTANCES			
		A	B	C	D
2.5" COMMON NAIL	0.128"	2-1/8"	1-5/8"	1-1/8"	5/8"
3.0" COMMON NAIL	0.144"	2-3/8"	1-3/4"	1-1/4"	5/8"
3.5" COMMON NAIL	0.160"	2-5/8"	2"	1-3/8"	3/4"
2.5" SPIRAL NAIL	0.109"	1-3/4"	1-3/8"	7/8"	1/2"
3.0" SPIRAL NAIL	0.122"	2"	1-1/2"	1"	1/2"
3.5" SPIRAL NAIL	0.152"	2-1/2"	1-7/8"	1-1/4"	5/8"
3.25" GUN NAIL	0.128"	2-1/8"	1-5/8"	1-1/8"	5/8"

++ - CSA 086-01 ALLOWABLES ROUNDED UP TO NEAREST 1/8"



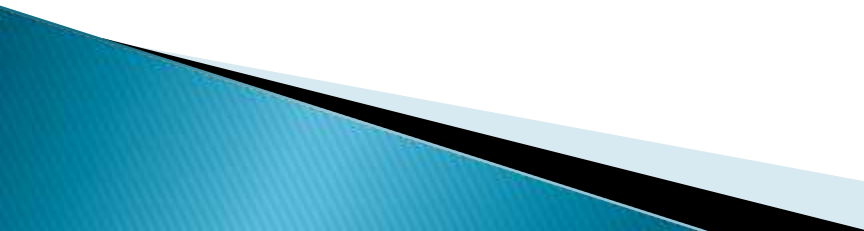
****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCST J-03 (HANDLING, INSTALLING AND BRACING), PUBLISHED BY THE TRUSS PLATE INSTITUTE, 265 DUNDAS ST. W., SUITE 200, MISSISSAUGA, ON L5R 4R9 FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ALPINE SYSTEMS CORPORATION SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSSES IN CONFORMANCE WITH THIS OR FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF CSA 086-01 (CANADIAN STANDARDS ASSOCIATION), NBCC (LATEST EDITION), AND TPIC ALPINE CONNECTORS ARE MADE OF 6061 T3 ALUMINUM OR 304 STAINLESS STEEL EXCEPT AS NOTED. APPLY CONNECTORS TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE NOTED ON THIS DESIGN, POSITION CONNECTORS PER DRAWINGS 96B 4-2. THE SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY PARTICULAR BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER. PER TPIC 96.



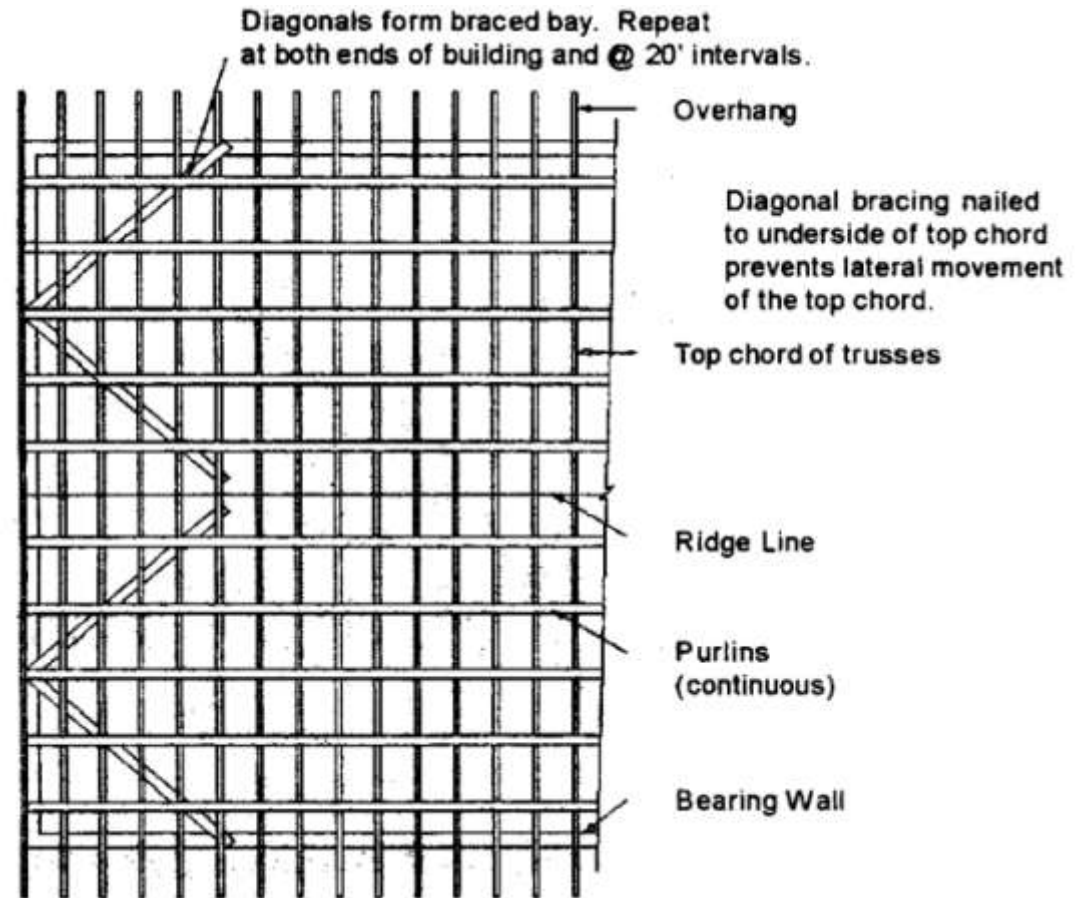
REF	BEARING BLOCK
DATE	12/04/03
DRWG	CNDRGBLK1103
-ENG	TB/AV

Building Bracing of Trusses

- ▶ Permanent bracing is designed and specified for the structural safety of the building. The building designer should indicate the size, location and attachments for all permanent bracing
 - ▶ The builder must still install the required permanent bracing, when missed by the building designer
 - ▶ Typical permanent bracing to be specified by the building designer is as follows:
- 

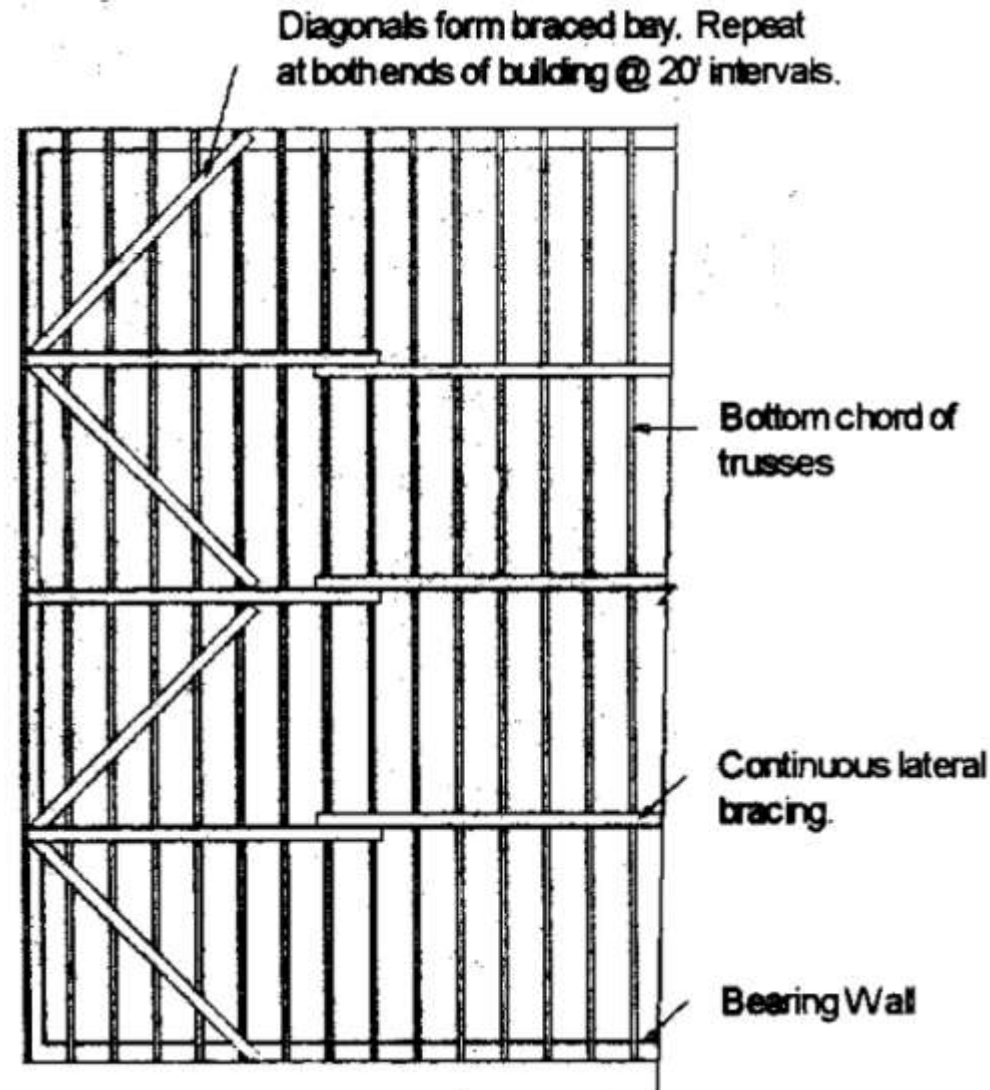
Top Chord Bracing – no rigid roof sheathing

- ▶ If purlins are used, it is recommended that diagonal bracing be applied to the underside of the top chord as shown.



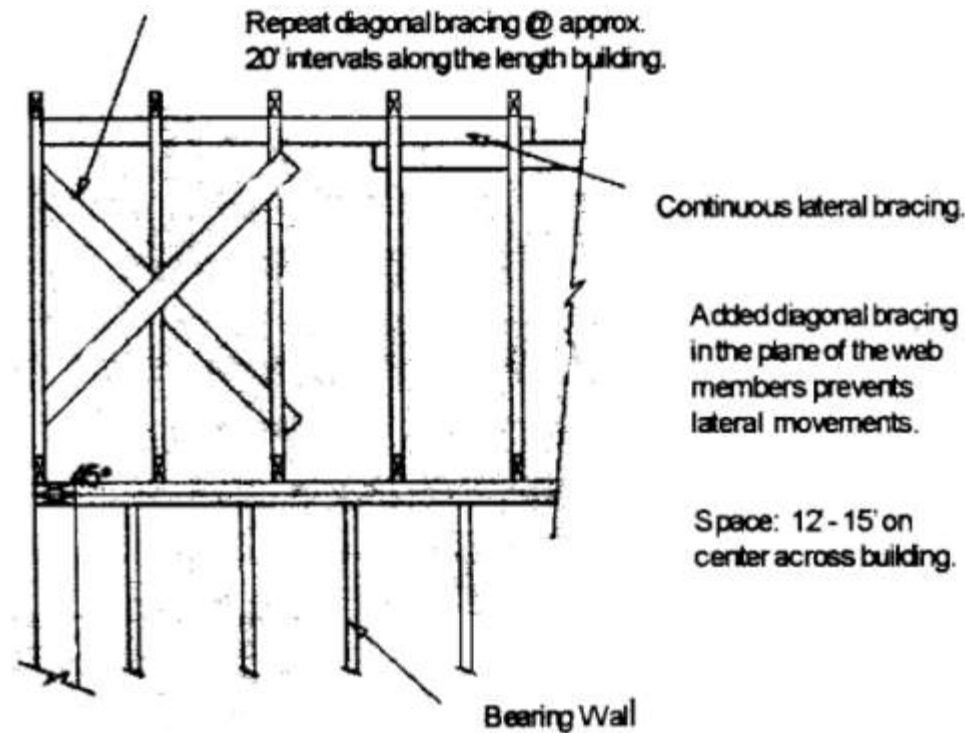
Bottom Chord Bracing – no rigid ceiling

- ▶ This lateral and diagonal bracing is required to maintain the proper truss spacing and to transfer force due to lateral forces into the side walls, shear walls or other resisting structural elements.



Diagonal Web Bracing

- ▶ The diagonal web bracing specified by the building designer is used to hold the trusses in a vertical position, to maintain the proper spacing, to distribute unequal loading to adjacent trusses and to transfer lateral forces to the diaphragms and shear walls.



Possible Problems

- ▶ Trusses not installed according to truss drawings (spacing, field connections, nailing of girders, bracing wrong or missing, trusses installed backwards or upside down)
- ▶ Eg. This truss arrived on site in 2 pcs and was installed similarly to 2x12 rafters (butted at the peak with blocking)

Installed Incorrectly

TOP CHORD 2x4 SPF No.1/No.2
 BOT CHORD 2x4 SPF 1650F-1.5E
 WEBS 2x4 SPF No.1/No.2

MAX CSI: TC = 0.22, BC = 0.33, WEBS = 0.28.

SEE A-101, NOTE 3 FOR STANDARD PLATE POSITIONING.

NIE AS PER CLAUSE 5.5.13.5 OF CSA-086-01 FOR TRUSS DESIGN.

PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.

4/8x PLATE POSITIONING TOLERANCE.

THIS DWG. PREPARED BY THE ALPINE JOB DESIGNER PROGRAM FROM TRUSS MFR'S LAYOUT

Loc	H	V	S	L	D	F	H _z
A 0'	8'11"2	3'9"	458	119	125	904	0
B 23'8"8	8'11"2	3'9"	396	119	119	804	0

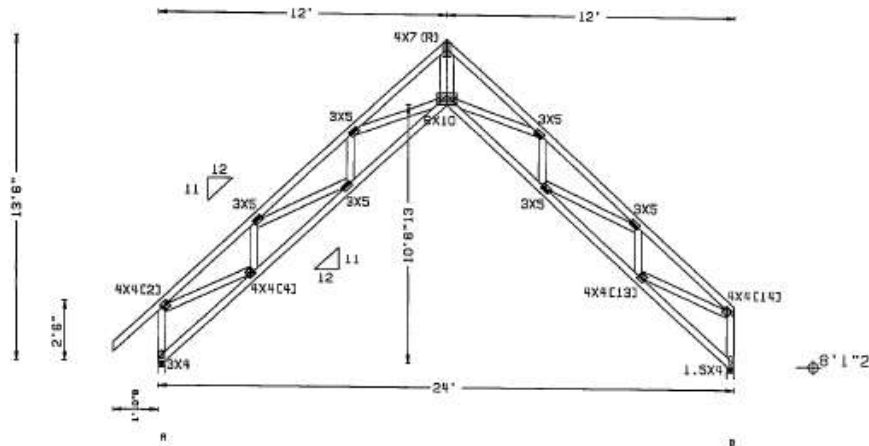
CALCULATED HORIZONTAL DEFLECTION OF 0.51" DUE TO LIVE LOAD AND 0.15" DUE TO DEAD LOAD AT ONE SUPPORT.

DEFLECTION MEETS L/360.00 TOTAL LOAD.

TRUSSES TO BE SPACED AT 12.0" O.C. MAXIMUM.

LOADING SPEC'D BY AUTH. HAVING JURISDICTION @ TIME OF DESIGN.

JT No	PLATE SIZE	LATERAL SHIFT	CHORD BITE
(2)	4X4	1.75	1.50
(4)	4X4	2.25	1.50
(13)	4X4	1.75	1.50
(14)	4X4	2.25	1.50



LEFT RAKE = 2'6"8

PLT. TYP.-WAVE-CANFOR

DESIGN CRIT-RESIDENTIAL

QTY= 15 TOTAL= 15

REV. 9.02.02.0514

SEO = 36961
 SCALE = 0.1667

THIS DRAWING MUST BE REVIEWED BY A REGISTERED PROFESSIONAL ENGINEER BEFORE USE. VISIT www.alpinesys.com/specs FOR THE LATEST INFORMATION AND WARNINGS AND SEE A100 FOR GENERAL NOTES, IMPORTANT SPECIFICATIONS AND WARNINGS. CCMC #12182-L, 12802-L, 13124-L

Ground Snow Load = 48.70 psf OR LESS

Rain Load = 6.26 psf OR LESS

(Cb = 0.55 Cw = 1.00 Cs = 1.00, Importance factor= 1.00)

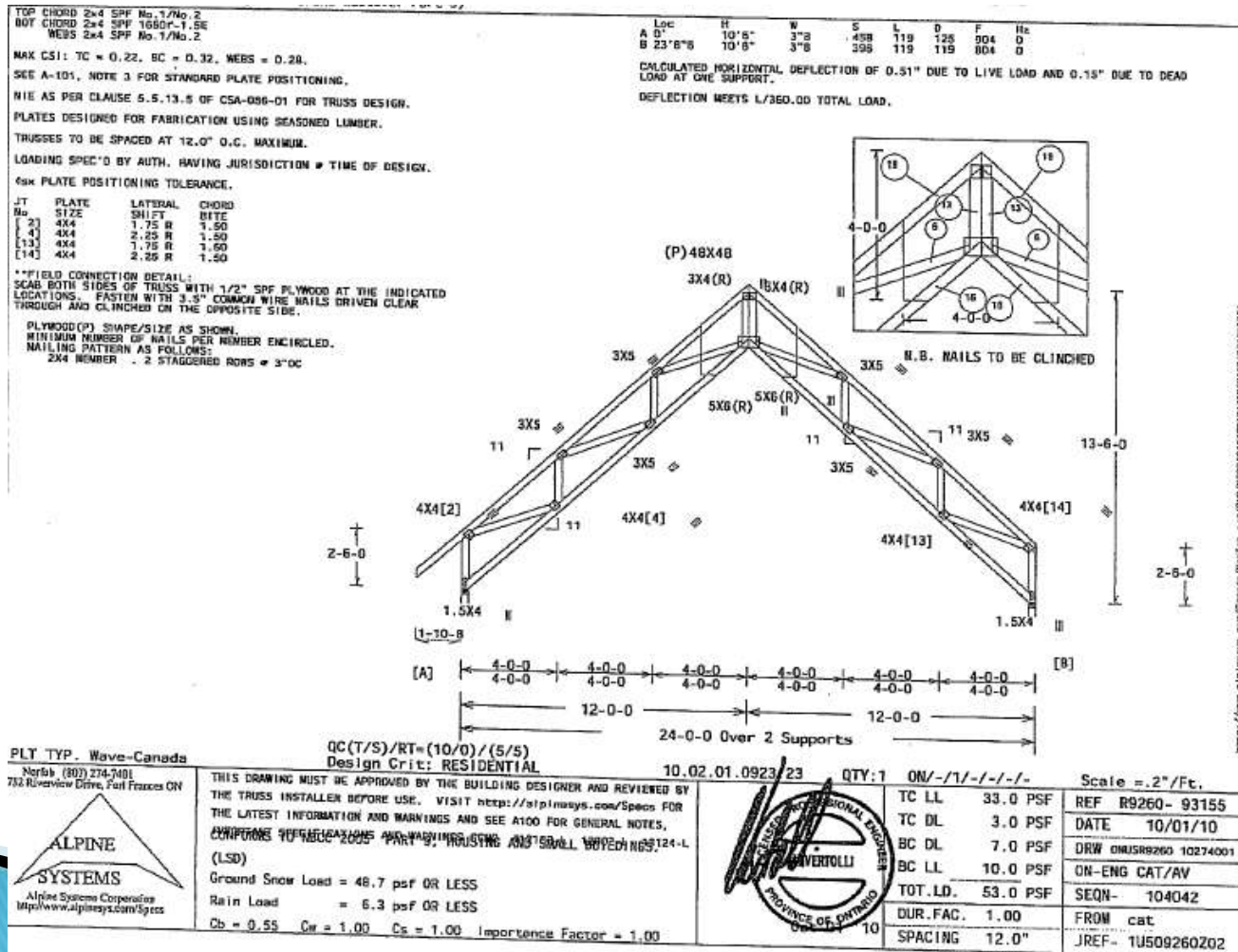
CONFORMS TO
 NBCC 2005 PART 9
 HOUSING AND
 SMALL BUILDINGS.

TC LL	33.0psf
TC DL	3.0psf
BC DL	7.0psf
BC LL	10.0psf
TOT.LO	53.0psf
DUR.FAC.	1.00
SPACING	12.0"

REF
DATE 09-08-2009
DRWG
CS
G/A LEN. 2*
JOB #: 29050
TYPE FLOS

Corrections

- ▶ Info missing on site, no field connection detail supplied to builder, add field connection




Corrections Cont'd

- ▶ Girder truss required a frame and bolts

<p>TOP CHORD: 2x4 SPF No.1/No.2 BOT CHORD: 2x4 SPF No.1/No.2 WEBS: 3x4 SPF No.1/No.2</p> <p>MAX CSI: TC = 0.28, BC = 0.38, WEBS = 0.25.</p> <p>SEE A-101, NOTE 3 FOR STANDARD PLATE POSITIONING.</p> <p>PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.</p> <p>CALCULATED HORIZONTAL DEFLECTION OF 0.48" DUE TO LIVE LOAD AND 0.11" DUE TO DEAD LOAD AT GIVE SUPPORT.</p> <p>LOADING SPEC'D BY ACTH. HAVING JURISDICTION • TIME OF DESIGN.</p> <p>4x4 PLATE POSITIONING TOLERANCE.</p> <p>PLATE SIZES INCREASED FOR 1.0GX REQ'D AREA AND STEEL SECTION.</p> <p>Resid.Ld[3SL]-3 (Lumber Dur.Fac.=1.00 / Plyge Dur.Fac.=1.00)</p> <table border="1"> <tr> <td>TC-</td> <td>0.00</td> <td>33/ 0/ 0/ 3</td> <td>12.00</td> <td>33/ 0/ 0/ 3</td> </tr> <tr> <td>TC-</td> <td>12.00</td> <td>83/ 0/ 0/ 8</td> <td>24.00</td> <td>33/ 0/ 0/ 3</td> </tr> <tr> <td>BC-</td> <td>0.00</td> <td>0/ 5/ 0/ 7</td> <td>12.00</td> <td>0/13/ 0/19</td> </tr> <tr> <td>BC-</td> <td>12.00</td> <td>0/13/ 0/18</td> <td>24.00</td> <td>0/ 5/ 0/ 7</td> </tr> </table> <p>DEFLECTION MEETS L/360 TOTAL LOAD.</p> <p>SITE CONNECTION:</p> <p>APPLY PRE-BUILT FRAMES TO BOTH SIDES OF THE TRUSS AS SHOWN.</p> <p>CONNECT ALL OVERLAPPING WEB MEMBERS WITH 3.5" COMMON WIRE OR GIR WAILS ϕ 4" OC:</p> <ul style="list-style-type: none"> 2X4 MEMBERS...2 STAGGERED BOWS 2X7 MEMBERS...4 STAGGERED BOWS <p>CONNECT ALL OVERLAPPING CHORD MEMBERS WITH 1/3" BOLTS ϕ 4" OC THROUGH CENTRELINE OF MEMBER.</p>	TC-	0.00	33/ 0/ 0/ 3	12.00	33/ 0/ 0/ 3	TC-	12.00	83/ 0/ 0/ 8	24.00	33/ 0/ 0/ 3	BC-	0.00	0/ 5/ 0/ 7	12.00	0/13/ 0/19	BC-	12.00	0/13/ 0/18	24.00	0/ 5/ 0/ 7	<p>3 COMPLETE TRUSSES REQUIRED</p> <p>Nail Schedule: 2 5" common nails TOP CHORD: 1 Row ϕ 4.00" o.c. BOT CHORD: 1 Row ϕ 6.00" o.c. Webs : 1 Row ϕ 4" o.c.</p> <p>Repeat nailing on each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.</p> <table border="1"> <tr> <td>Loc</td> <td>H</td> <td>W</td> <td>S</td> <td>L</td> <td>D</td> <td>F</td> <td>Na</td> </tr> <tr> <td>A 0'</td> <td>9'</td> <td>3"8</td> <td>695</td> <td>209</td> <td>215</td> <td>1418</td> <td>0</td> </tr> <tr> <td>B 23'6"8</td> <td>9'</td> <td>3"8</td> <td>695</td> <td>209</td> <td>215</td> <td>1418</td> <td>0</td> </tr> </table>	Loc	H	W	S	L	D	F	Na	A 0'	9'	3"8	695	209	215	1418	0	B 23'6"8	9'	3"8	695	209	215	1418	0											
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BC-	12.00	0/13/ 0/18	24.00	0/ 5/ 0/ 7																																																				
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B 23'6"8	9'	3"8	695	209	215	1418	0																																																	
<p>Note: All Plates Are 3X5 Except As Shown.</p> <p>QC(T/S)/RT=(10/0)/(10/10) Design Crit: RESIDENTIAL</p>																																																								
<table border="1"> <tr> <td>PLT TYP. Wave-Canada</td> <td>10.02.02.10.23</td> <td>QTY:1</td> <td>DN/-/1/-/-/-</td> <td>Scale = 2"/Ft.</td> </tr> <tr> <td>Norfolk (907) 274-2441 322 Riverside Drive Fort Ferrier ON</td> <td colspan="3">THIS DRAWING MUST BE APPROVED BY THE BUILDING DESIGNER AND REVIEWED BY THE TRUSS INSTALLER BEFORE USE. VISIT http://alpineeng.com/Specs FOR THE LATEST INFORMATION AND DRAWINGS AND SEE A100 FOR GENERAL NOTES. CONFORMS TO THE CANADIAN PROVISIONS OF THE NATIONAL BUILDING CODE (NBC) AND THE CANADIAN STANDARD CODES FOR ALUMINUM (LSD)</td> <td>REF R9260- 11692</td> </tr> <tr> <td rowspan="2"> <p>Alpine Systems Corporation www.alpineeng.com</p> </td> <td colspan="3">Ground Snow Load = 48.7 psf OR LESS</td> <td>DATE 10/28/10</td> </tr> <tr> <td colspan="3">Rain Load = 6.3 psf OR LESS</td> <td>DRW ONUSRS250 10501002</td> </tr> <tr> <td colspan="4">Cb = 0.55 Cw = 1.00 Cs = 1.00 Importance Factor = 1.00</td> <td>ON-ENG TB/AV</td> </tr> <tr> <td colspan="4"> </td> <td>SEQN- 134731 REV</td> </tr> <tr> <td colspan="4"> <table border="1"> <tr> <td>TC LL</td> <td>33.0 PSF</td> <td>FROM RAG</td> </tr> <tr> <td>TC DL</td> <td>3.0 PSF</td> <td>JREF- 1UG9260Z01</td> </tr> <tr> <td>BC DL</td> <td>7.0 PSF</td> <td></td> </tr> <tr> <td>BC LL</td> <td>10.0 PSF</td> <td></td> </tr> <tr> <td>TOT L.D.</td> <td>53.0 PSF</td> <td></td> </tr> <tr> <td>DUR.FAC.</td> <td>1.00</td> <td></td> </tr> <tr> <td>SPACING</td> <td>24.0"</td> <td></td> </tr> </table> </td> <td></td> </tr> </table>		PLT TYP. Wave-Canada	10.02.02.10.23	QTY:1	DN/-/1/-/-/-	Scale = 2"/Ft.	Norfolk (907) 274-2441 322 Riverside Drive Fort Ferrier ON	THIS DRAWING MUST BE APPROVED BY THE BUILDING DESIGNER AND REVIEWED BY THE TRUSS INSTALLER BEFORE USE. VISIT http://alpineeng.com/Specs FOR THE LATEST INFORMATION AND DRAWINGS AND SEE A100 FOR GENERAL NOTES. CONFORMS TO THE CANADIAN PROVISIONS OF THE NATIONAL BUILDING CODE (NBC) AND THE CANADIAN STANDARD CODES FOR ALUMINUM (LSD)			REF R9260- 11692	<p>Alpine Systems Corporation www.alpineeng.com</p>	Ground Snow Load = 48.7 psf OR LESS			DATE 10/28/10	Rain Load = 6.3 psf OR LESS			DRW ONUSRS250 10501002	Cb = 0.55 Cw = 1.00 Cs = 1.00 Importance Factor = 1.00				ON-ENG TB/AV					SEQN- 134731 REV	<table border="1"> <tr> <td>TC LL</td> <td>33.0 PSF</td> <td>FROM RAG</td> </tr> <tr> <td>TC DL</td> <td>3.0 PSF</td> <td>JREF- 1UG9260Z01</td> </tr> <tr> <td>BC DL</td> <td>7.0 PSF</td> <td></td> </tr> <tr> <td>BC LL</td> <td>10.0 PSF</td> <td></td> </tr> <tr> <td>TOT L.D.</td> <td>53.0 PSF</td> <td></td> </tr> <tr> <td>DUR.FAC.</td> <td>1.00</td> <td></td> </tr> <tr> <td>SPACING</td> <td>24.0"</td> <td></td> </tr> </table>				TC LL	33.0 PSF	FROM RAG	TC DL	3.0 PSF	JREF- 1UG9260Z01	BC DL	7.0 PSF		BC LL	10.0 PSF		TOT L.D.	53.0 PSF		DUR.FAC.	1.00		SPACING	24.0"		
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Design Problem

- ▶ Trusses designed incorrectly by manufacturer (usually incorrect loading, wrong type of building or wrong bearing type)
 - ▶ Eg: This example shows a truss design mistake
 - ▶ During a site visit the trusses were on site, so I asked to see the truss drawings
 - ▶ Can you spot the problem?
- 

	Loo	H	W	S	L	D	F	Hz
A	0'	8'	5'-8"	1037	279	291	2080	1326
B	27'-6"-8"	8'	5'-8"	1037	279	291	2080	1326

TRUSSES TO BE PROPERLY ANCHORED AT SUPPORTS, BY OTHERS, TO WITHSTAND THE INDICATED VERTICAL AND HORIZONTAL REACTIONS.

DEFLECTION MEETS L/360.00 TOTAL LOAD.

In lieu of structural walls on field columns use steel...

The 1326 pound horizontal reaction indicates that this truss is designed to sit on a continuous floor which will prevent the horizontal forces from the truss forcing the walls out. On site, there is no floor for these trusses to sit on. The design is not appropriate for the building.

Correction: New trusses required!

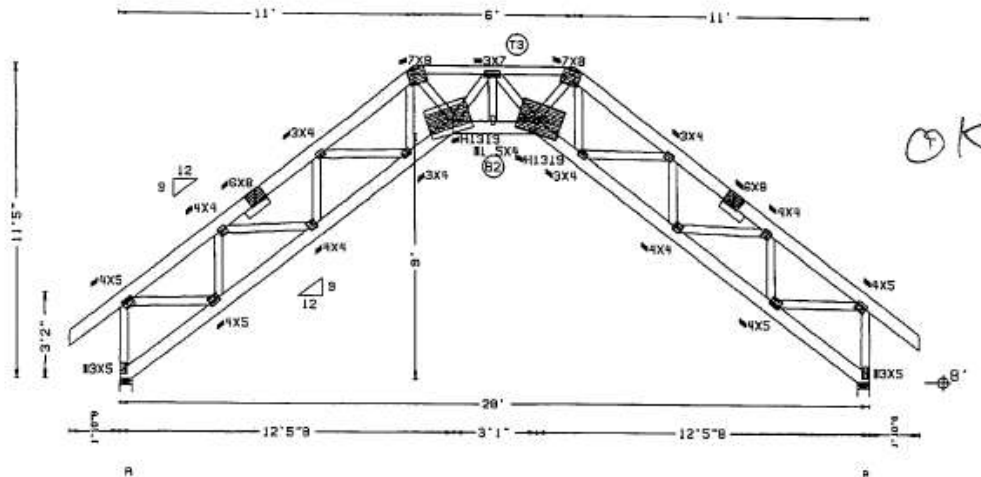
TOP CHORD 2x6 SPF No.1/No.2 :T3 2x4 SPF 2100F-1.8E:
 JOIST CHORD 2x6 SPF No.1/No.2 :B2 2x6 SPF 1850F-1.13E:
 WEBS 2x4 SPF No.1/No.2 :Rt Splice Block 2x4 SPF No.1/No.2:
 Lt Splice Block 2x4 SPF No.1/No.2:Rt Splice Block 2x4 SPF No.1/No.2:
 MAX CSI: TC = 0.39, BC = 0.68, WEBS = 0.69.
 SEE R-101, NOTE 3 FOR STANDARD PLATE POSITIONING.
 NIE AS PER CLAUSE 5.5.13.5 OF CSA-086-01 FOR TRUSS DESIGN.
 PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.

THIS SPECIFICATION AND COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

L	1048	L	279	D	291	F	2077	H ₂	0
H	8'	S	1048	L	279	D	291	F	2077
B	27'6"8	V	5"8	L	279	D	291	F	2077

CALCULATED HORIZONTAL DEFLECTION OF 0.58" DUE TO LIVE LOAD AND 0.16" DUE TO DEAD LOAD AT ONE SUPPORT.
 DEFLECTION MEETS L/360.00 TOTAL LOAD.
 IN LIEU OF STRUCTURAL PANELS USE PURLINS TO BRACE ALL FLAT TO @ 24.00" OC.
 LOADING SPEC'D BY AUTH. HAVING JURISDICTION @ TIME OF DESIGN.
 1/8" PLATE POSITIONING TOLERANCE.

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LEFT RAKE = 2'4"2

RIGHT RAKE = 2'4"2

PLT. TYP.-WAVE-CANADA	DESIGN CRIT-RESIDENTIAL	QTY- 14 TOTAL- 14	REV. 9.05.02.0303	SEQ - 118608 SCALE = 0.1875
THIS DRAWING MUST BE REVIEWED BY A REGISTERED PROFESSIONAL ENGINEER BEFORE USE. VISIT www.alpineys.com/specs FOR THE LATEST INFORMATION AND WARNINGS AND SEE ALSO FOR GENERAL NOTES, IMPORTANT SPECIFICATIONS AND WARNINGS. CCMC #12182-L, 12902-L, 13124-L		CONFORMS TO NBCC 2005 PART 9 HOUSING AND SMALL BUILDINGS.	TC LL ✓ 33.0psf	REF
Ground Snow Load = 48.70 psf OR LESS ✓			TC DL ✓ 3.0psf	DATE 10-14-2010
Rain Load = 6.26 psf OR LESS ✓			BC DL ✓ 7.0psf	DRWG
(Cb = 0.55 Cw = 1.00 Cs = 1.00, Importance Factor = 1.00) ✓			BC LL ✓ 10.0psf	ARG
			TOT.LD ✓ 53.0psf	Q/A LEN. 29
			DUR.FAC ✓ 1.00	JOB #: 29888
			SPACING ✓ 24.0"	TYPE SCIH

Other Code Issues

- ▶ Roof insulation – minimum R12 at inside face of exterior walls
 - ▶ Roof Venting – minimum 1” clearance space when using pre-formed baffles, 2.5” otherwise
 - ▶ The result of the above – minimum truss depth at the exterior wall must be 8” to 12” depending on the insulation used
- 

Floor Trusses – Take care to check

- ▶ Loads, spacing, bearing locations, hangers
- ▶ Lateral bracing 2x6 strong backs
- ▶ All specified on truss drawings

Floor Truss Drawing

TOP CHORD 4x2 SPF 2100f-1.8E
 BOT CHORD 4x2 SPF 2100f-1.8E
 WEBS 4x2 SPF No.1/No.2

MAX CSI: TC = 0.61, BC = 0.49, WEBS = 0.45.

PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.

PLATE SIZES INCREASED FOR 1.05X REQ'D AREA AND STEEL SECTION.

JT No	PLATE SIZE	LATERAL SHIFT	CHORD BITE
[1]	3X4	1.50 L	1.25
[2]	4X10	2.75 L	1.25
[5]	4X12	5.75 L	1.25
[7]	3X5	1.50 R	1.25
[10]	3X5	1.50 L	1.25
[13]	4X12	5.75 R	1.25
[14]	4X10	2.75 R	1.25
[17]	3X4	1.50 R	1.25

REFER TO DRAWING A150 FOR TYPICAL PLATE LOCATIONS.

DEFLECTION MEETS L/480.00 LIVE LOAD AND L/240.00 TOTAL LOAD.

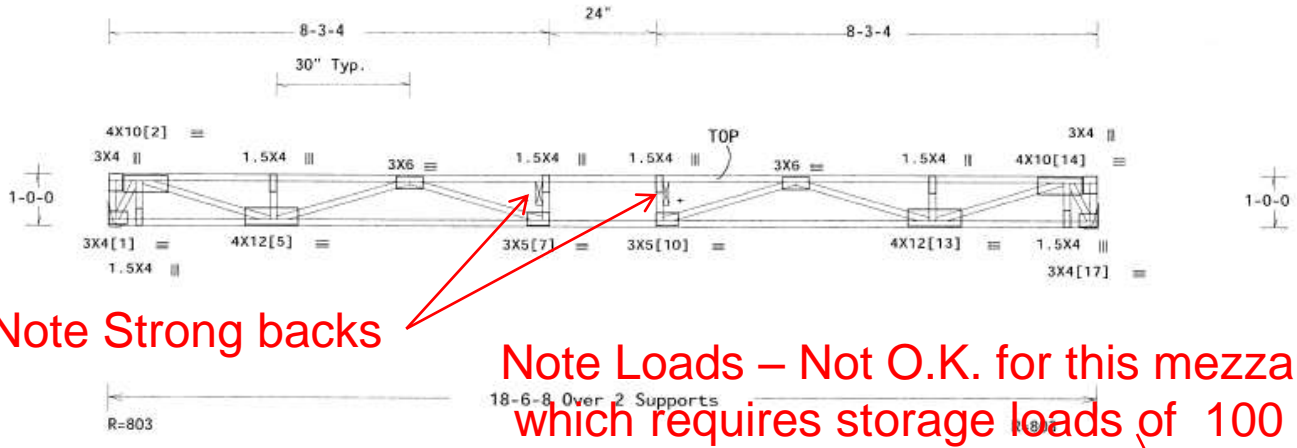
TRUSSES TO BE SPACED AT 16.0" O.C. MAXIMUM.

NOTE: THIS TRUSS MUST BE INSTALLED AS SHOWN. IT CANNOT BE USED UPSIDE DOWN. TOP OF TRUSS MUST BE MARKED BY TRUSS FABRICATOR.

FOR HANGER SELECTION, L/S FACTORED TL REACTION = 1159#. FOR HANGER SELECTION, R/S FACTORED TL REACTION = 1159#.

FLOOR TRUSS MEETS CCMC VIBRATION REPORT DATED SEPT. 12, 1997 WITH THE FOLLOWING FLOOR ASSEMBLY, SUBFLOOR MATERIAL SPF, SUBFLOOR THICKNESS 0.63". FLOOR ATTACHMENT NAILED AND GLUED, STRONGBACKS 2x6SPF#1/#2 ATTACHED TO EACH TRUSS W/3-10d NAILS. MAT. SUPPL'D BY ERECTION CONTR. LOC AS SHOWN. NO CONCRETE FLOOR TOPPING. NO CEILING ATTACHED TO TRUSS.

Hangers Rx



Note Strong backs

Note Loads – Not O.K. for this mezzanine floor which requires storage loads of 100 psf LL

PLT TYP. Wave-Canada
 Norfab (807) 274-7401
 732 Riverview Drive, Fort Frances ON

Alpine Systems Corporation
<http://www.alpineys.com/Specs>

Design Crit: FLOOR 7.31.1205.22 QTY: 14 AB/-/1/1-1-1 Scale = .375"/Ft.

THIS DRAWING MUST BE APPROVED BY THE BUILDING DESIGNER AND REVIEWED BY THE TRUSS INSTALLER BEFORE USE. VISIT <http://www.alpineys.com/Specs> FOR THE LATEST INFORMATION AND WARNINGS AND SEE A100 FOR GENERAL NOTES. IMPORTANT SPECIFICATIONS AND WARNINGS CCMC #12182-L 12802-L 13124-L CONFORMS TO NBCC 1995 PART 4, FLOOR TRUSS DESIGN (LSD)

FLOOR LOADING AS SHOWN



TC LL	50.0 PSF	REF	R9260- 86053
TC DL	0.0 PSF	DATE	07/03/07
BC DL	5.0 PSF	DRW	0NUSR9260 07184001
BC LL	0.0 PSF	ON-ENG	DS/CT
TOT. LD.	65.0 PSF	SEQN-	45461
DUR. FAC.	1.00	FROM	RM
SPACING	16.0"	JREF-	1T8K9260Z01

http://www.alpineys.com/Canada/NorFab.nsf/00E48034901841E8237000507430

Correction: Add Trusses to get 8"o.c

TOP CHORD 4x2 SPF 2100F 1.8E
 BOT CHORD 4x2 SPF 2100F 1.8E
 WEBS 4x2 SPF No.1/No.2

MAX CSI: TC = 0.55, BC = 0.44, WEBS = 0.40.

PLATES DESIGNED FOR FABRICATION USING SEASONED LUMBER.

PLATE SIZES INCREASED FOR 1.05X RED'D AREA AND STEEL SECTION.

JT No	PLATE SIZE	LATERAL SHIFT	CHORD BITE
[1]	3x4	1.50 L	1.25
[2]	4x10	3.25 L	1.25
[7]	3x5	1.50 R	1.25
[10]	3x5	1.50 L	1.25
[14]	4x10	3.25 R	1.25
[17]	3x4	1.50 R	1.25

REFER TO DRAWING ALSO FOR TYPICAL PLATE LOCATIONS.

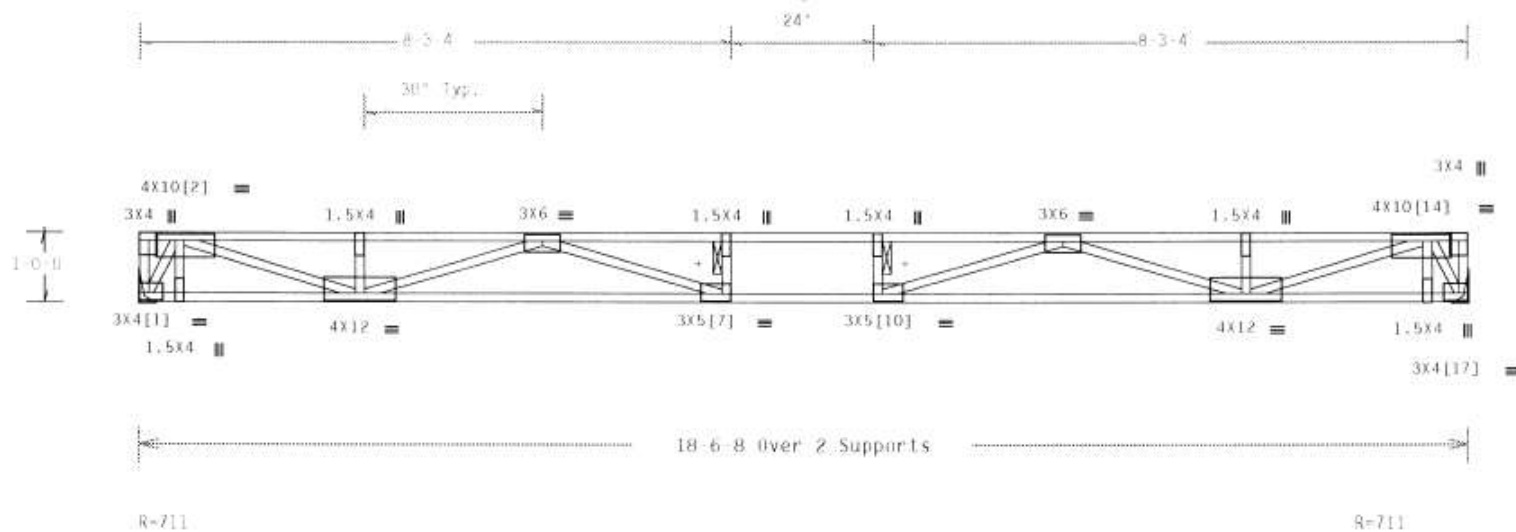
DEFLECTION MEETS L7480100 LIVE LOAD AND L1560100 TOTAL LOAD.

TRUSSES TO BE SPACED AT 8.0" O.C. MAXIMUM.

NOTE: THIS TRUSS MUST BE INSTALLED AS SHOWN; IT CANNOT BE USED UPSIDE DOWN. TOP OF TRUSS MUST BE MARKED BY TRUSS FABRICATOR.

FOR HANGER SELECTION, L/S FACTORED TL REACTION = 1043#. FOR HANGER SELECTION, R/S FACTORED TL REACTION = 1043#.

FLOOR TRUSS MEETS OCMC VIBRATION REPORT DATED SEPT. 12, 1997 WITH THE FOLLOWING FLOOR ASSEMBLY: SUBFLOOR MATERIAL SPF, SUBFLOOR THICKNESS 0.65", FLOOR ATTACHMENT NAILED AND GLUED, STRINGRACKS 2x6SPF#1/#2 ATTACHED TO EACH TRUSS W/3-10d NAILS. MAT. SUPPL'D BY ERECTION CONTR. LOC AS SHOWN. NO CONCRETE FLOOR TOPPING, NO CEILING ATTACHED TO TRUSS.



http://www.alpine.com/Canada/vertab_nsf/0001761749841744926855330004/5899

PLT TYP. Wave Canada

Design Crit: FLOOR

7.35.0313.22

QTY:14 ON/1/1/1/1/1/1

Scale = .45"/Ft.

Norfab (807) 274-7401
 732 Rossview Drive, Fort Frances ON

THIS DRAWING MUST BE APPROVED BY THE BUILDING DESIGNER AND REVIEWED BY THE TRUSS INSTALLER BEFORE USE. VISIT <http://alpinewe.com/links> FOR THE LATEST INFORMATION AND WARNINGS AND SEE A100 FOR GENERAL NOTES. IMPORTANT SPECIFICATIONS AND WARNINGS OCMC #12192 L 18902 L 18124 L CONFORMS TO NEC 1995 PART 4, FLOOR TRUSS DESIGN (LSD)



TC LL	100.0 PSF	REF R9260-43134
TC DL	10.0 PSF	DATE 08/23/07
BC DL	5.0 PSF	DRW 08092607 07265001
BC LL	0.0 PSF	ON-ENG DS/UC
TOT. LD.	115.0 PSF	SEQN- 10967 REV
DUR.FAC.	1.00	FROM PM
SPACING	8.0"	JREF- 11A59260701

FLOOR LOADING AS SHOWN




Truss Installation: Summary

- ▶ Examine Trusses on site for correct types, quantities, hangers, damage, stamped truss drawings and layout drawings.
- ▶ Arrange for Hoisting, review all drawings, assemble bracing materials
- ▶ Make note of trusses which appear symmetrical but may not be. Such as cantilevers, interior bearings, point loads, etc. –these must be installed correctly
- ▶ Check for proper orientation of flat trusses – do not install upside down or with wrong end placement

Truss Installation: Summary

- ▶ Decide if multi-ply trusses will be nailed together on the ground or when on the walls
- ▶ Mark out truss locations on wall plates according to truss and layout drawings – never exceed the required truss spacing
- ▶ Install trusses plumb with straight tops and bottoms – 3 Nails per bearing (3-1/4”), or on supplied hangers, add temporary and permanent bracing to secure the trusses, do not nail trusses to partitions
- ▶ Attach purlins as specified or use sheathing

Truss Installation: Summary

- ▶ On trusses without rigid ceilings, install bottom chord bracing as specified in drawings
 - ▶ Install web bracing as specified in drawings
 - ▶ Install permanent building bracing in trusses as specified by building designer or as discussed earlier
 - ▶ Do not add concentrated loads to trusses (no lifts of plywood, no large stacks of shingles)
 - ▶ Avoid damage to trusses by mechanical contractors, excessive moisture
- 

Questions?

Recommendations?

Stories?

Ideas?

